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This file or an individual page shall not be considered a certified document.

STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS MACON COUNTY S <u>1310</u> LOCATION: BRIDGE NO.17 OVER WAYAH CREEK ON <u> 1437</u> SR 1309 (CRAWFORD RD.) B TYPE OF WORK: GRADING, DRAINAGE, PAVING & STRUCTURE PROJECT SITE END BRIDGE BEGIN TIP PROJECT B-4775 -L- POC STA. 14 + 55.67 -L-POTSTA.13+00.00VICINITY MAP OFF-SITE DETOUR → ◆ ◆ ◆ (CARL SLAGLE RD) **_L**_ **SR** 1309 (CRAWFORD RD.) BEGIN BRIDGE -L-POC STA. 14+08.33END TIP PROJECT B-4775 S -L-POC STA. 15+60.00END CONSTRUCTION -L- POC STA. 16 + 00.00 STRUCTURE PLANS PROJECT LENGTH DESIGN DATA DIVISION OF HIGHWAYS DIVISION 14 ADT 2016 = 1101223 Jones Franklin Rd. Raleigh, N.C. 2760 253 Webster Road. Sylva, NC 28801 License No. F-0377 Bus: 919.851.8077 Fax: 919.851.8107 T = 6 % *2018 STANDARD SPECIFICATIONS LENGTH ROADWAY TIP PROJECT B-4775 = 0.040 MILES V = 25 MPHEDWARD G. WETHERILL, PE = 0.009 MILES LENGTH STRUCTURE PROJECT B-4775 * TTST = 3% DUAL = 3% FUNC. CLASS = LOCAL TOTAL LENGTH TIP PROJECT B-4775 = 0.049MILES LETTING DATE: B.C. HUNT, P.E. (SUBREGIONAL) 2018 PROJECT DESIGN ENGINEER JOSH DEYTON, PE NCDOT CONTACT:

B-4775 BRZ-1309(8) 38546.1.1 BRZ-1309(8) ROW/UTIL. 38546.1.1

BRIDGE #17

DIVISION 14 BRIDGE PROGRAM MANAGER

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

S-1

TOTAL SHEETS

NO. BY:

BY:

DATE:

DATE:

1223 Jones Franklin Rd. Raleigh, N.C. 27606 Bus: 919 851 8077 Fax: 919 851 8107 LICENSE NO. F-0377

DOCUMENT NOT CONSIDERED FINAL

UNLESS ALL SIGNATURES COMPLETE

(SPREAD FOOTINGS & PILES NOT SHOWN IN PLAN VIEW)

DRAWN BY: B.C. HUNT / MJS DATE: 1-2017

CHECKED BY: J.A. DILWORTH DATE: 1-2017

NOTES:

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 400 TONS OF REINFORCING STEEL, TWO 30 INCH SAMPLES OF EACH SIZE BAR USED. THE BARS FROM WHICH THE SAMPLES ARE TAKEN MUST THEN BE SPLICED WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF THE SAMPLE, PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS. PAYMENT FOR THE SAMPLES OF REINFORCING STEEL SHALL BE CONSIDERED INCIDENTAL TO VARIOUS PAY

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INLCUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE AT STATION 13+38.00 -L-."

THE MATERIAL SHOWN IN THE HATCHED AREA SHALL BE EXCAVATED FOR A DISTANCE OF 30 FT. EACH SIDE OF CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

THE EXISTING STRUCTURE CONSISTING OF 1 SPAN @ 30'-6" AND A CLEAR ROADWAY WIDTH OF 19'-1", A TIMBER DECK COVERED WITH ASPHALT, AND SUPPORTED BY STEEL I BEAMS AND A SUBSTRUCTURE OF TIMPER CAPS AND POSTS ON CONCRETE FOOTINGS SHALL BE REMOVED.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18 -EVALUATING SCOUR AT BRIDGES".

FOR EROSION CONTROL MEASURES SEE EROSION CONTROL PLANS.

ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY

FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.

							1	OTAL	BILL O	F MATEI	RIAL —						
	REMOVAL OF EXISTING STRUCTURE	FOUNDATION EXCAVATION FOR END BENT	UNCLASSIFIED STRUCTURE EXCAVATION	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	HP STEI	12 X 53 EL PILES	PILE REDRIVES	STEEL PILE POINTS	PILE DRIVING EQUIPMENT SETUP FOR HP 12×53 STEEL PILES	VERTICAL CONCRETE BARRIER RAIL	RIP RAP CLASS II (2'-0"THICK)	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	3'-0" x 1'-9" PRESTRESSED CONCRETE CORED SLABS	ASBESTOS ASSESSMENT
	LUMP SUM	LUMP SUM	LUMP SUM	CU. YDS.	LUMP SUM	LBS.	NO.	LIN.FT.	EACH	EACH	EACH	LIN.FT.	TONS	SQ. YDS.	LUMP SUM	No. LIN.FT.	LUMP SUM
SUPERSTRUCTURE												90.00			LUMP SUM	9 405.00	
END BENT 1		LUMP SUM		28.6		4,703							60	65			
END BENT 2				19.3		2,326	5	100	3	5	5		45	50			
TOTAL	LUMP SUM	LUMP SUM	LUMP SUM	47.9	LUMP SUM	7,029	5	100	3	5	5	90.00	105	115	LUMP SUM	9 405.00	LUMP SUM

THE SPREAD FOOTINGS AT END BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 9 TSF.CHECK FIELD CONDITIONS FOR THE REQUIRED RESISTANCE OF 20 TSF JUST BEFORE PLACING CONCRETE.

KEY IN SPREAD FOOTINGS AT END BENT NO.1 AT LEAST 12" INTO ROCK WITH MINIMUM THICKNESS AS SHOWN ON THE

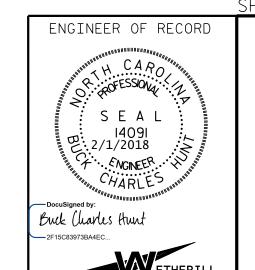
FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

PILES AT END BENT NO.2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 70 TONS PER PILE.

DRIVE PILES AT END BENT NO.2 TO A REQUIRED DRIVING RESISTANCE OF 120 TONS PER PILE.

STEEL H-PILE POINTS ARE REQUIRED FOR STEEL H-PILES AT END BENT NO.2. FOR STEEL PILE POINTS, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

IT HAS BEEN ESTIMATED THAT A HAMMER WITH AN EQUIVALENT RATED ENERGY IN THE RANGE OF 12,000 TO 25,000 FT-LBS PER BLOW WILL BE REQUIRED TO DRIVE PILES AT END BENT NO. 2. THIS ESTIMATED ENERGY RANGE DOES NOT RELEASE THE CONTRACTOR FROM PROVIDING DRIVING EQUIPMENT IN ACCORDINCAE WITH SUBARTICLE 450-3(D)(2) OF THE STANDARD SPECIFICATIONS.



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

B-4775

COUNTY

GENERAL DRAWING BRIDGE ON SR 1309 OVER WAYAH CREEK BETWEEN SR 1307 AND SR 1470

SHEET NO REVISIONS S-2 NO. BY: DATE: DATE: SHEETS

DRAWN BY : B.C. HUNT / MJS DATE : 1-2017 CHECKED BY : J.A. DILWORTH DATE : 1-2017

DOCUMENT NOT CONSIDERED FINAL JNLESS ALL SIGNATURES COMPLETE

FOUNDATION NOTES:

MACON

PROJECT NO._

STATION: 14+32.00 -L-

SHFFT 2 OF 2

W ETHERILL

1223 Jones Franklin Rd.

Raleigh, N.C. 27606 Bus: 919 851 8077

Fax: 919 851 8107

LICENSE NO. F-0377

BY:

										STRE	ENGTH	I LIN	1IT S	TATE				SE	ERVICE	III	LIMI	T STA	TE	
										MOMENT					SHEAR						MOMENT			
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W X RF	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f+)	COMMENT NUMBER
		HL-93(Inv)	N/A	1	1.098		1.75	0.272	1.36	45′	EL	21.982	0.617	1.46	45′	EL	35.172	0.80	0.272	1.10	45′	EL	21.982	
DESIGN		HL-93(0pr)	N/A		1.764		1.35	0.272	1.76	45′	EL	21.982	0.617	1.89	45′	EL	35.172	N/A						
LOAD RATING		HS-20(Inv)	36.000	2	1.347	48.507	1.75	0.272	1.67	45′	EL	21.982	0.617	1.68	45′	EL	8.793	0.80	0.272	1.35	45′	EL	21.982	
NATING		HS-20(0pr)	36.000		2.165	77.938	1.35	0.272	2.16	45′	EL	21.982	0.617	2.17	45′	EL	8.793	N/A						
		SNSH	13.500		2.632	35.536	1.4	0.272	4.08	45′	EL	21.982	0.617	4.43	45′	EL	35.172	0.80	0.272	2.63	45′	EL	21.982	
		SNGARBS2	20.000		2.126	42.513	1.4	0.272	3.29	45′	EL	21.982	0.617	3.32	45′	EL	35.172	0.80	0.272	2.13	45′	EL	21.982	
		SNAGRIS2	22.000		2.085	45.877	1.4	0.272	3.19	45′	EL	17.586	0.617	3.15	45′	EL	35.172	0.80	0.272	2.09	45′	EL	21.982	
	>	SNCOTTS3	27.250		1.314	35.814	1.4	0.272	2.04	45′	EL	21.982	0.617	2.23	45′	EL	8.793	0.80	0.272	1.31	45′	EL	21.982	
	S	SNAGGRS4	34.925		1.16	40.51	1.4	0.272	1.8	45′	EL	21.982	0.617	1.97	45′	EL	35.172	0.80	0.272	1.16	45′	EL	21.982	
		SNS5A	35.550		1.13	40.167	1.4	0.272	1.75	45′	EL	21.982	0.617	2.06	45′	EL	8.793	0.80	0.272	1.13	45′	EL	21.982	
		SNS6A	39.950		1.064	42.522	1.4	0.272	1.65	45′	EL	21.982	0.617	1.94	45′	EL	35.172	0.80	0.272	1.06	45′	EL	21.982	
LEGAL		SNS7B	42.000	3	1.015	42.617	1.4	0.272	1.57	45′	EL	21.982	0.617	1.98	45′	EL	35.172	0.80	0.272	1.01	45′	EL	21.982	
LOAD RATING		TNAGRIT3	33.000		1.306	43.112	1.4	0.272	2.02	45′	EL	21.982	0.617	2.26	45′	EL	8.793	0.80	0.272	1.31	45′	EL	21.982	
IVATINO		TNT4A	33.075		1.32	43.663	1.4	0.272	2.05	45′	EL	21.982	0.617	2.14	45′	EL	35.172	0.80	0.272	1.32	45′	EL	21.982	
		TNT6A	41.600		1.108	46.093	1.4	0.272	1.72	45′	EL	21.982	0.617	2.11	45′	EL	35.172	0.80	0.272	1.11	45′	EL	21.982	
	TS.	TNT7A	42.000		1.129	47.436	1.4	0.272	1.75	45′	EL	21.982	0.617	1.96	45′	EL	35.172	0.80	0.272	1.13	45′	EL	21.982	
		TNT7B	42.000		1.176	49.384	1.4	0.272	1.82	45′	EL	21.982	0.617	1.88	45′	EL	35.172	0.80	0.272	1.18	45′	EL	21.982	
		TNAGRIT4	43.000		1.12	48.157	1.4	0.272	1.74	45′	EL	21.982	0.617	1.8	45′	EL	35.172	0.80	0.272	1.12	45′	EL	21.982	
		TNAGT5A	45.000		1.042	46.893	1.4	0.272	1.61	45′	EL	21.982	0.617	1.88	45′	EL	35.172	0.80	0.272	1.04	45′	EL	21.982	
		TNAGT5B	45.000		1.017	45.785	1.4	0.272	1.58	45′	EL	21.982	0.617	1.7	45′	EL	35.172	0.80	0.272	1.02	45′	EL	21.982	

LOAD FACTORS:

LIMIT STATE γ_{DC} DESIGN LOAD RATING 1.25 STRENGTH I FACTORS SERVICE III | 1.00 | 1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

(#) CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

3 LEGAL LOAD RATING **

** SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

I - INTERIOR GIRDER

EL - EXTERIOR LEFT GIRDER

ER - EXTERIOR RIGHT GIRDER

PROJECT NO. B-4775

MACON COUNTY

STATION: 14+32.00 -L-

ENGINEER OF RECORD S E A L |409| |2/1/2018 EMANDER CHARLES Buck Charles Hunt ETHERILL ENGINEERING

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD LRFR SUMMARY FOR 45'CORED SLAB UNIT 105° SKEW

(NON-INTERSTATE TRAFFIC)

SHEET NO. REVISIONS S-3 DATE: NO. BY: DATE: TOTAL SHEETS

LRFR SUMMARY

FOR SPAN 'A'

ASSEMBLED BY: J. PENDERGRAFT DATE: 1-17 CHECKED BY: J. DILWORTH DATE: 1-17

DRAWN BY: CVC 6/10 CHECKED BY: DNS 6/10

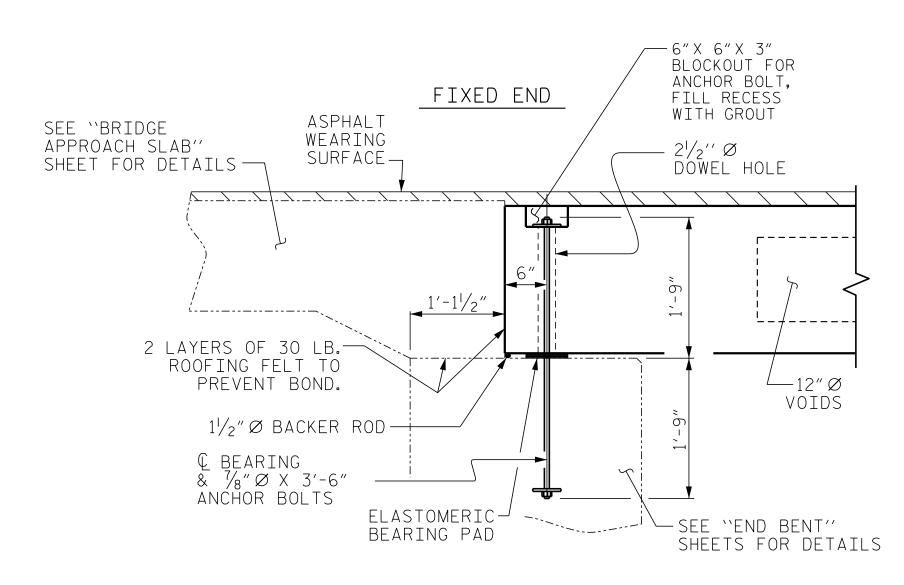
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

1223 Jones Franklin Rd. Raleigh, N.C. 27606 Bus: 919 851 8077 Fax: 919 851 8107 LICENSE NO. F-0377 STD. NO. 21LRFR1_75&105S_45L HALF SECTION
THROUGH VOIDS

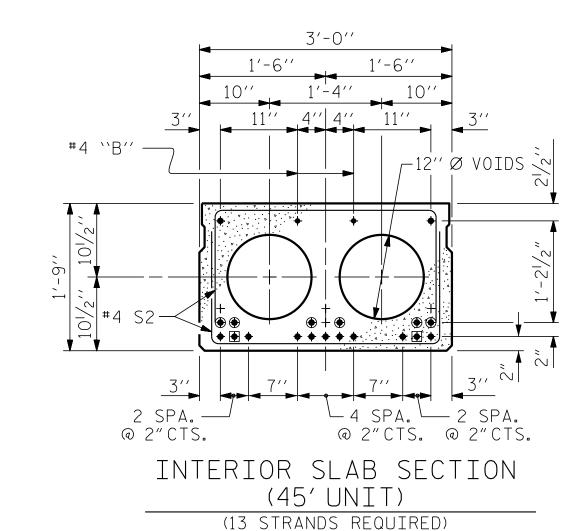
HALF SECTION
AT INTERMEDIATE DIAPHRAGMS

TYPICAL SECTION

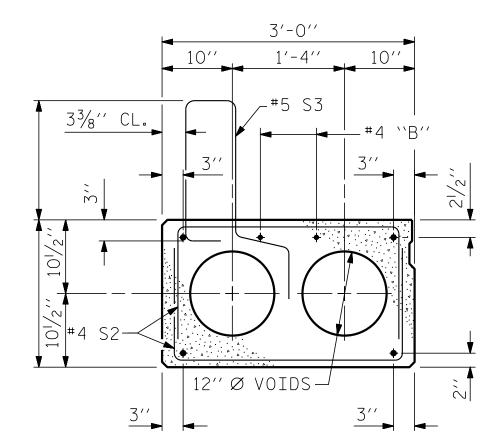
*-THE MAXIMUM BARRIER RAIL HEIGHT AND ASPHALT THICKNESS IS SHOWN. THE HEIGHT OF THE BARRIER RAIL AND ASPHALT THICKNESS VARIES WHILE THE TOP OF THE BARRIER RAIL FOLLOWS THE PROFILE OF THE GUTTERLINE. FOR RAIL HEIGHT DETAILS AND ASPHALT THICKNESS SEE THE "VERTICAL CONCRETE BARRIER RAIL SECTION" DETAIL.



SECTION AT END BENT



0.6" Ø LOW RELAXATION STRAND LAYOUT



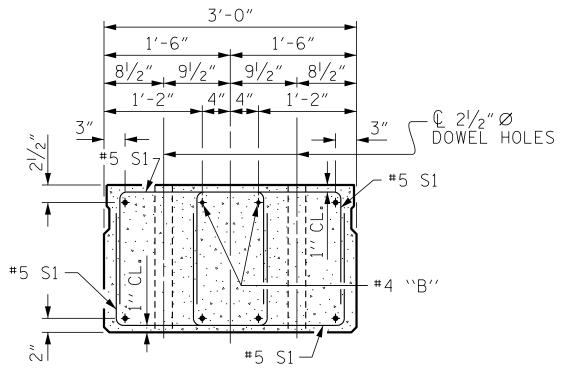
EXT. SLAB SECTION

(FOR PRESTRESSED STRAND LAYOUT, SEE INTERIOR SLAB SECTION.)

BOND SHALL BE BROKEN ON THESE STRANDS FOR A DISTANCE OF 2'-O" FROM END OF CORED SLAB UNIT. SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.

OPTIONAL FULL LENGTH DEBONDED STRANDS.
THESE STRANDS ARE NOT REQUIRED. IF THE
FABRICATOR CHOOSES TO INCLUDE THESE STRANDS
IN THE CORED SLAB UNIT, THE STRANDS SHALL
BE DEBONDED FOR THE FULL LENGTH OF THE UNIT
AT NO ADDITIONAL COST. SEE STANDARD
SPECIFICATIONS, ARTICLE 1078-7.

DEBONDING LEGEND

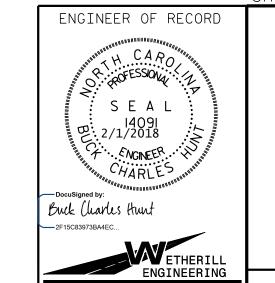


END ELEVATION

SHOWING PLACEMENT OF DOUBLE STIRRUPS
AND LOCATION OF DOWEL HOLES.
(STRAND LAYOUT NOT SHOWN.)
INTERIOR SLAB UNIT SHOWN-EXTERIOR SLAB
UNIT SIMILAR EXCEPT SHEAR KEY LOCATION.

PROJECT NO. _____B-4775 _____MACON ____county STATION: ___14+32.00 -L-

SHEET 1 OF 4



1223 Jones Franklin Rd. Raleigh, N.C. 27606 Bus: 919 851 8077 Fax: 919 851 8107

LICENSE NO. F-0377

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

STANDARD

3'-0'' X 1'-9''
PRESTRESSED CONCRETE
CORED SLAB UNIT
105° SKEW

REVISIONS

BY: DATE: NO. BY: DATE: S-4

3 TOTAL SHEETS
18

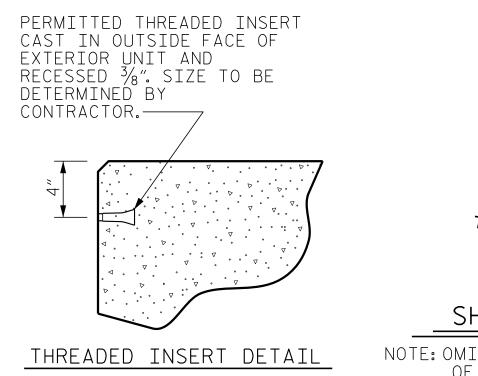
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DRAWN BY: J. PENDERGRAFT DATE: 1-17
CHECKED BY: J. DILWORTH DATE: 1-17

STD. NO. 21" PCS2_27_105S

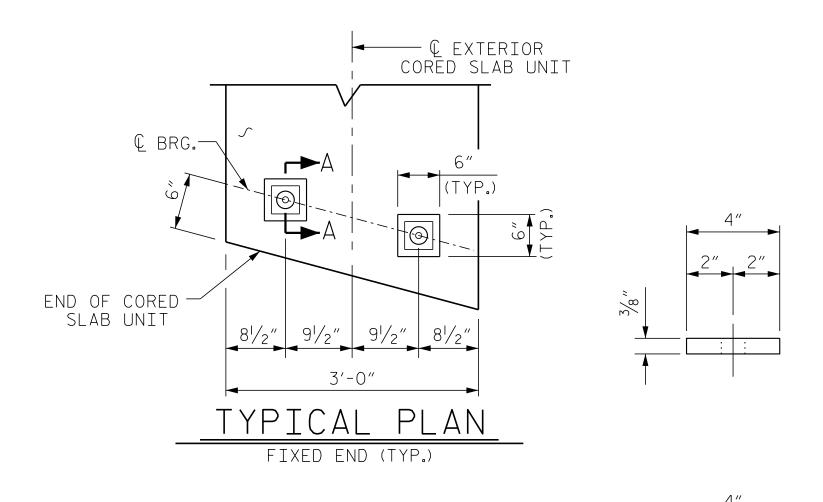
| 58.01_B-4775\Structures\DGN\MACON 17 TS.dgn

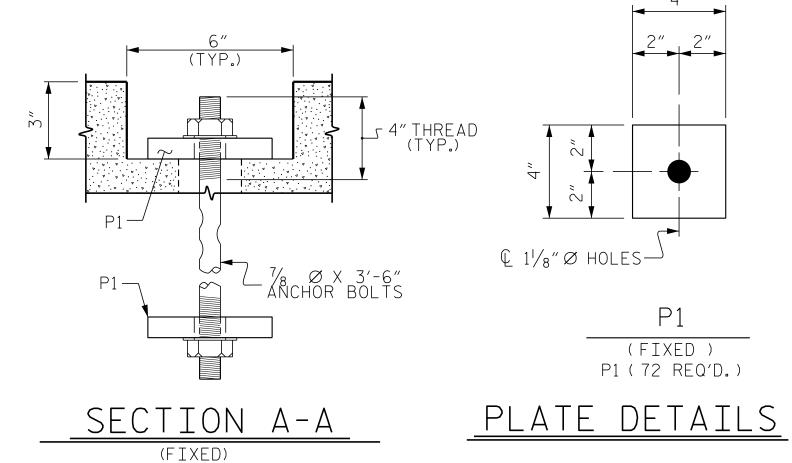
GROUTED RECESS AT END OF POST-TENSIONED STRAND OF CORED SLABS



NOTES:

% % ANCHOR BOLTS, NUTS, AND PLATE WASHERS FOR CORED SLAB UNITS SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.





BLOCKOUT DETAIL FOR ANCHOR BOLTS

PROJECT NO. B-4775 MACON COUNTY STATION: 14+32.00 -L-

SHEET 2 OF 4 ENGINEER OF RECORD LA CAROLINA RESSON S E A L |409| |2/1/2018 THE CHARLES WARREN Buk Charles Hunt

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

STANDARD

3'-0'' X 1'-9''
PRESTRESSED CONCRETE
CORED SLAB UNIT
105° SKEW

SHEET NO. REVISIONS S-5 NO. BY: DATE: BY: TOTAL SHEETS

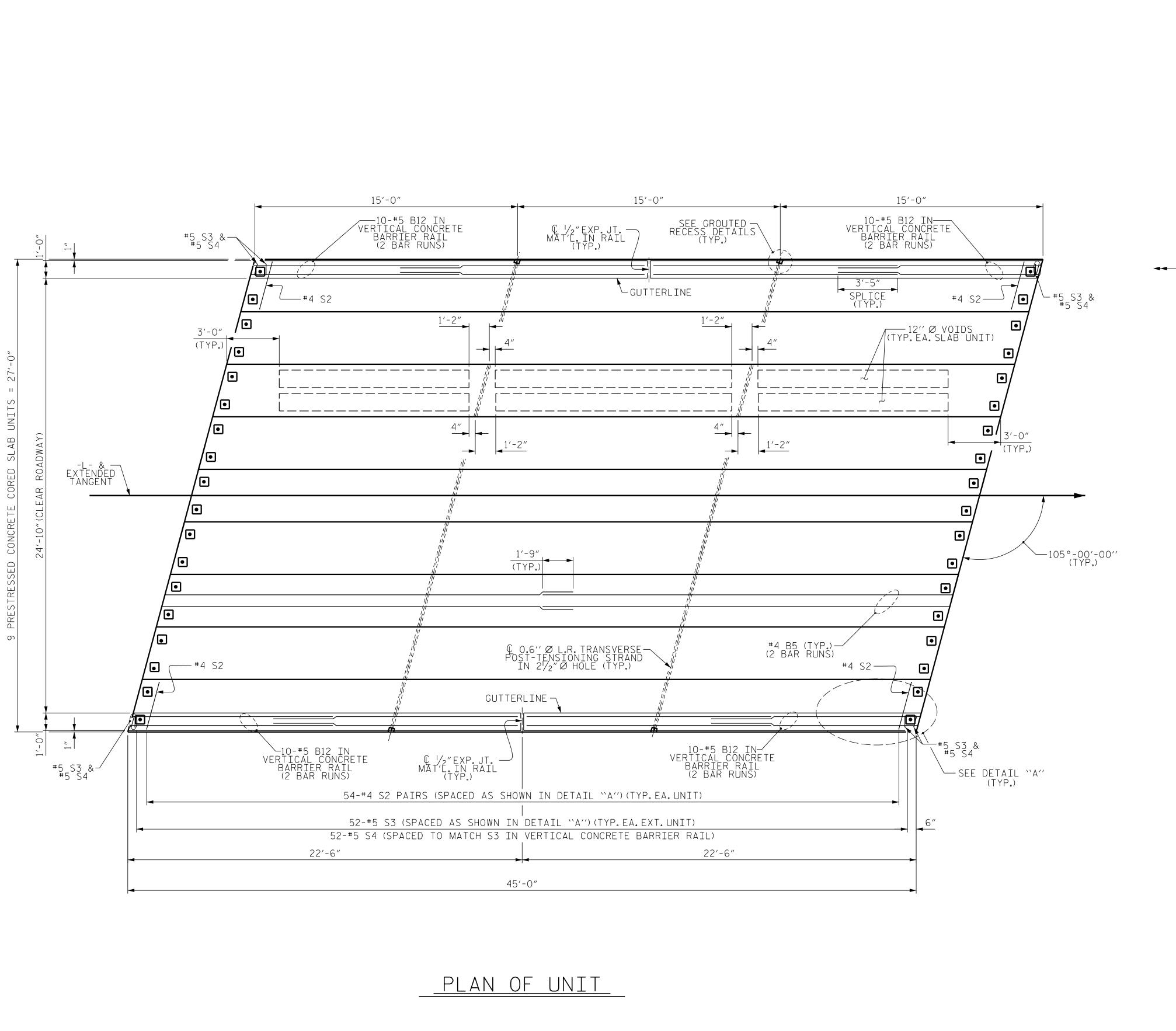
ENGINEERING 1223 Jones Franklin Rd. Raleigh, N.C. 27606 Bus: 919 851 8077 Fax: 919 851 8107 LICENSE NO. F-0377

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

_ DATE : <u>1-17</u> _ DATE : ________ CHECKED BY : B.C. HUNT

SHEAR KEY DETAIL

NOTE: OMIT SHEAR KEY ON OUTSIDE FACE OF EXTERIOR CORED SLABS.



6"X 6"X 3"
BLOCKOUT EACH
END (EXTERIOR
UNITS ONLY)

5'-4|3/6"

Q 2/2" Ø
DOWEL HOLES

#5 S3 @ 1'-0"CTS. 8-#5 S3 @ 6"CTS. 6"

#4 S2 PAIRS
@ 1'-0"CTS. @ APPROX. EQ. SPA.

(SIMILAR EACH END OF UNIT) NOTE: EXTERIOR UNIT SHOWN - INTERIOR UNIT SIMILAR EXCEPT OMIT #5 S3 BARS

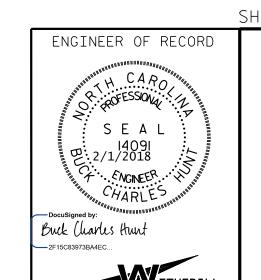
DETAIL "A"

PROJECT NO. B-4775

MACON COUNTY

STATION: 14+32.00 -L-

SHEET 3 OF 4



1223 Jones Franklin Rd. Raleigh, N.C. 27606 Bus: 919 851 8077 Fax: 919 851 8107 LICENSE NO. F-0377 DEPARTMENT OF TRANSPORTATION
RALEIGH

PLAN OF 45'UNIT 24'-10"CLEAR ROADWAY 105° SKEW

REVISIONS SHEET NO.

BY: DATE: NO. BY: DATE: S-6

TOTAL SHEETS

18

DRAWN BY: J. PENDERGRAFT DATE: 1-17
CHECKED BY: J. DILWORTH DATE: 1-17

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

ELASTOMERIC BEARING DETAILS

ELASTOMER IN ALL BEARINGS SHALL BE 50 DUROMETER HARDNESS.

 $3'-0'' \times 1'-9$

0.6" Ø L.R.

STRAND

1/8"

3/4"

DEAD LOAD DEFLECTION AND CAMBER

45' CORED SLAB UNIT

CAMBER (SLAB ALONE IN PLACE

** INCLUDES FUTURE WEARING SURFACE

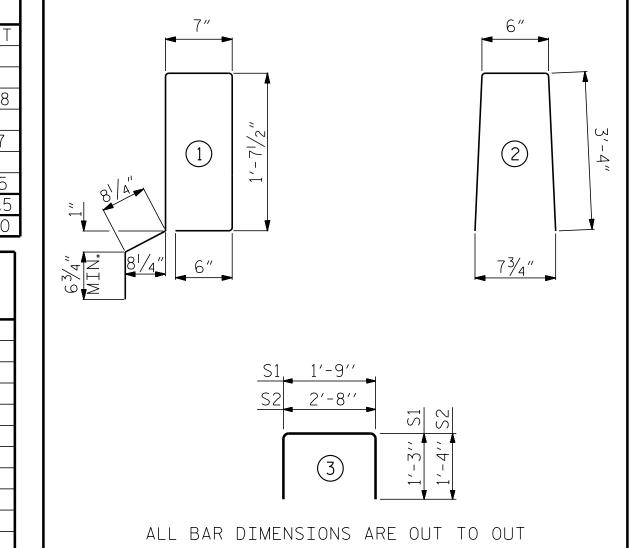
SUPERIMPOSED DEAD LOAD**

DEFLECTION DUE TO

FINAL CAMBER

ВІ	ILL OF MATERIAL FOR VERTI	CAL CONC	RETE	BARR	RIER R	AIL
BAR	BARS PER PAIR OF EXTERIOR UNITS	TOTAL NO.	SIZE	TYPE	LENGTH	WEIGHT
	45' UNIT					
 ₩ B12	80	80	#5	STR	12'-11"	1078
* S4	108	108	#5	2	7′-2″	807
* EPOX	(Y COATED REINFORCING STEEL			LBS.		1885
CLASS	AA CONCRETE			CU.YDS.	1	11.5
TOTAL	VERTICAL CONCRETE BARRIER RAIL		_	LN. FT.		90.00

				MATERIA RED SLAE		NE		
				EXTERI	OR UNIT	INTERIOR UNIT		
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT	
B5	4	#4	STR	23'-3"	62	23'-3"	62	
S1	8	#5	3	4'-3"	35	4'-3"	35	
S2	108	#4	3	5′-4″	385	5′-4″	385	
* S3	54	#5	1	5′-7″	314			
REINFO	ORCING S	STEEL	LBS	5.	482		482	
* EPOXY COATED REINFORCING STEEL LBS. 31								
5000 F	P.S.I.CO	NCRETE	CU. YDS) a	6.6		6.6	
0.6"Ø	L.R. STR	ANDS	No) .	13		13	



BAR TYPES

CORED SLABS REQUIRED							
	NUMBER	LENGTH	TOTAL LENGTH				
45' UNIT							
EXTERIOR C.S.	2	45′-0″	90'-0"				
INTERIOR C.S.	7	45′-0″	315′-0″				
TOTAL	9		405'-0"				

ASPHALT OVERLAY THICKNESS

@ MID-SPAN

GUTTERLINE ASPHALT THICKNESS & RAIL HEIGHT

4-#5 S3 6" 4-#5 S3

45' UNITS

CONCRETE	RELEAS	SE STRE	NGTH
UNIT		PSI	
45' UNITS		4000)

GRADE 270 S	TRANDS
	0.6″∅ L.R.
AREA (SQUARE INCHES)	0.217
ULTIMATE STRENGTH (LBS.PER STRAND)	58,600
APPLIED PRESTRESS (LBS.PER STRAND)	43,950

RAIL HEIGHT

@ MID-SPAN

3′-8″

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE 21/2" Ø DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

ALL REINFORCING STEEL IN THE VERTICAL CONCRETE BARRIER RAIL SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

GROOVED CONTRACTION JOINTS, $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE "CONCRETE RELEASE STRENGTH" TABLE.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.

THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-O"CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.

THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK.

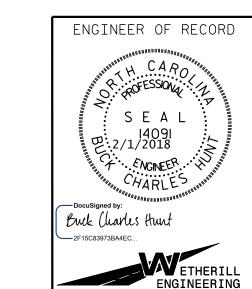
THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.

THE COST OF THE HOLD DOWN PLATES, ANCHOR BOLTS, NUTS, AND WASHERS SHALL BE INCLUDED IN THE CORED SLAB PAY ITEMS.

> B-4775 PROJECT NO._ MACON COUNTY

14+32.00 -L-STATION:

SHEET 4 OF 4



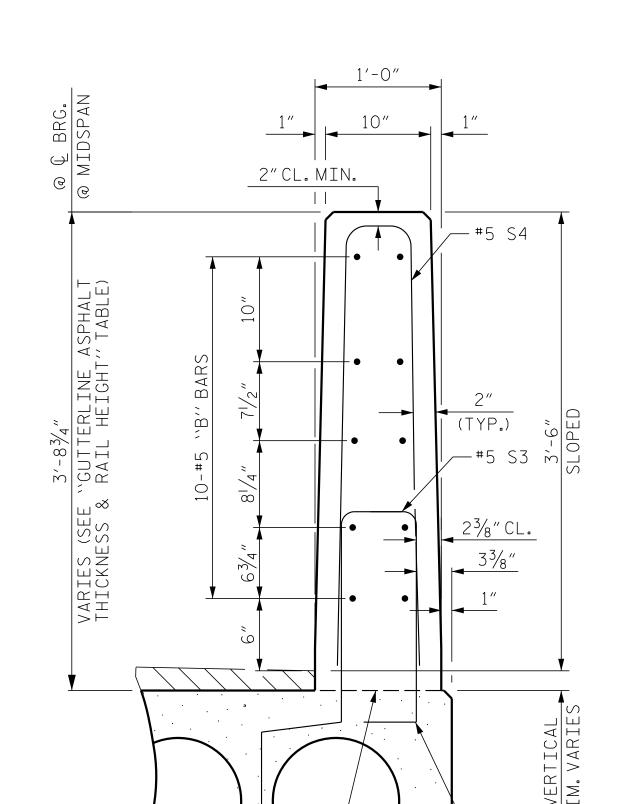
1223 Jones Franklin Rd.

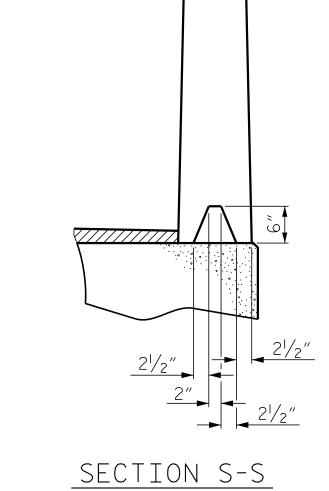
Raleigh, N.C. 27606 Bus: 919 851 8077 Fax: 919 851 8107

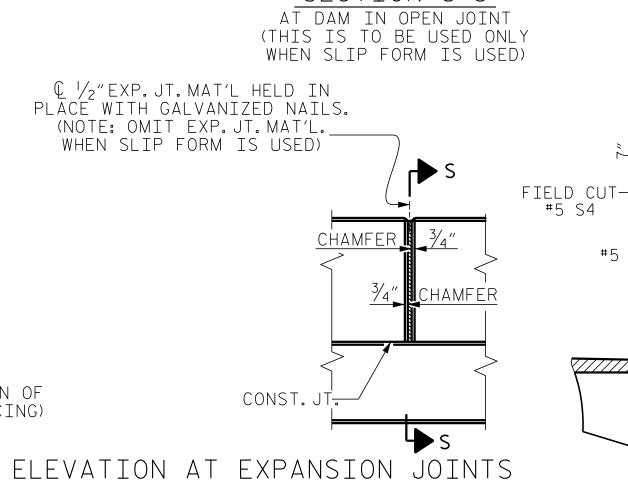
DEPARTMENT OF TRANSPORTATION STANDARD PRESTRESSED CONCRETE CORED SLAB UNIT

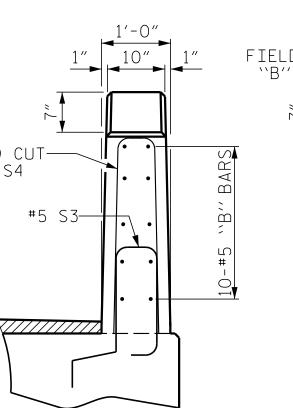
STATE OF NORTH CAROLINA

REVISIONS SHEET NO S-7 DATE: DATE: NO. BY: TOTAL SHEETS









#5 S<u>3 & S4</u> | & S4 @ `& S4 @ FIELD BEND — "B" BARS 6"CTS. \|FIELD CUT CUT #5 S4 CONST. JT.→

END VIEW

SIDE VIEW

DRAWN BY : J. PENDERGRAFT _ DATE : <u>1-17</u> CHECKED BY : J. DILWORTH _ DATE : <u>1-17</u>

CONST. JT. —

UNLESS ALL SIGNATURES COMPLETE

#5 S3 (SEE "PLAN OF UNIT" FOR SPACING)

VERTICAL CONCRETE BARRIER RAIL SECTION

AT DAM IN OPEN JOINT (THIS IS TO BE USED ONLY WHEN SLIP FORM IS USED) € 1/2" EXP. JT. MAT'L HELD IN

PLACE WITH GALVANIZED NAILS. (NOTE: OMIT EXP. JT. MAT'L. WHEN SLIP FORM IS USED) CHAMFE CONST. J

END OF RAIL DETAILS

DOCUMENT NOT CONSIDERED FINAL

CORED	SLABS	S REQ	UIRED
	NUMBER	LENGTH	TOTAL LENGTH
45' UNIT			
EXTERIOR C.S.	2	45'-0"	90'-0"
INTERIOR C.S.	7	45'-0"	315′-0″
TOTAL	9		405′-0″

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $1/4^{\prime\prime}$ HOLD DOWN PLATE AND 7 - $1/8^{\prime\prime}$ Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE $\frac{7}{8}$ GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

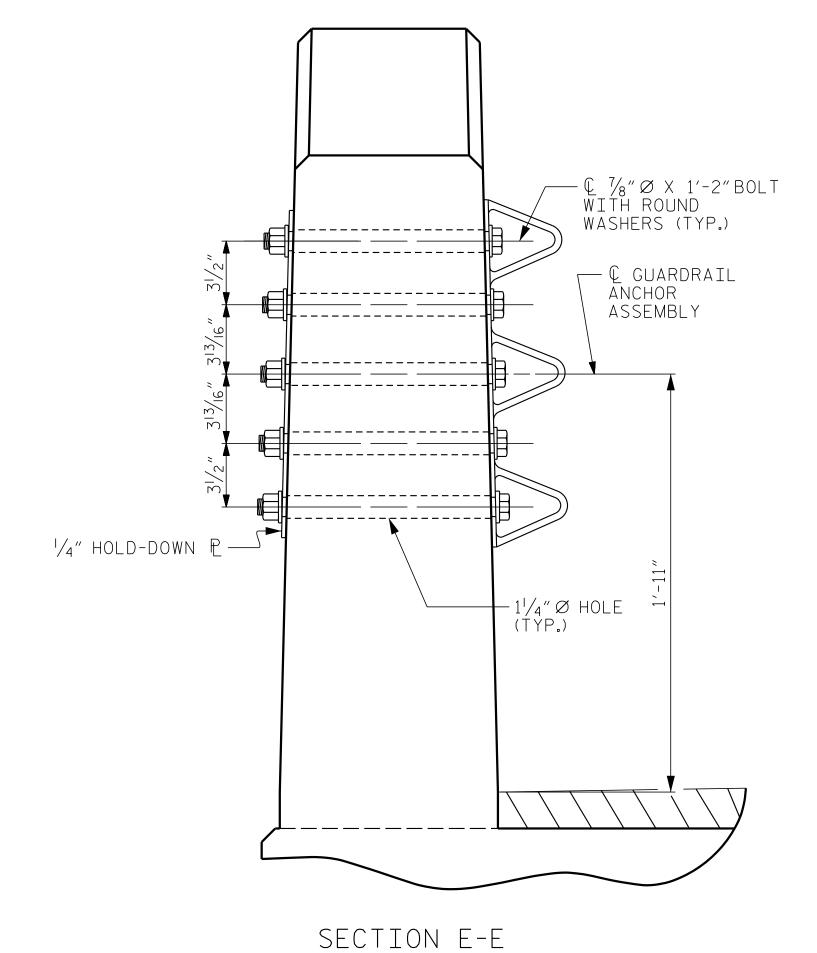
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

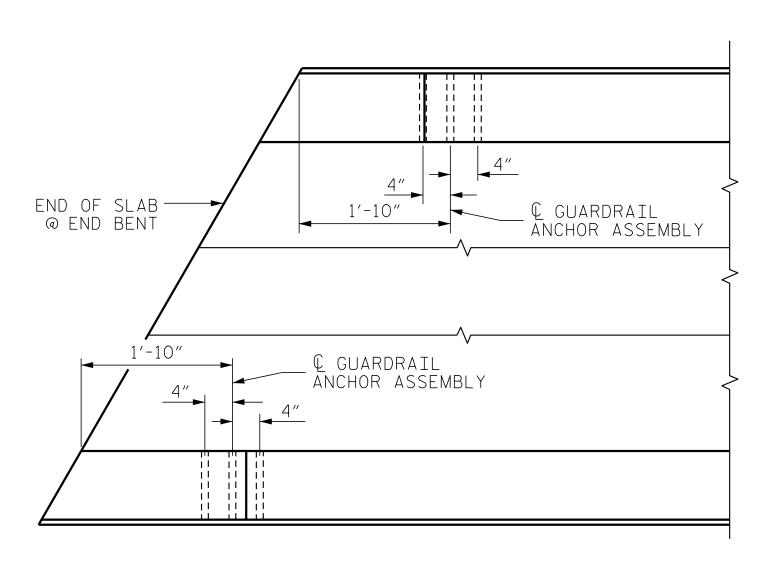
THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

THE 1 $\frac{1}{4}$ " Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.



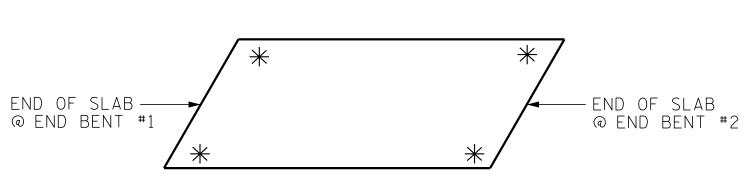
GUARDRAIL ANCHOR ASSEMBLY DETAILS



PLAN

LOCATION OF ANCHORS FOR GUARDRAIL

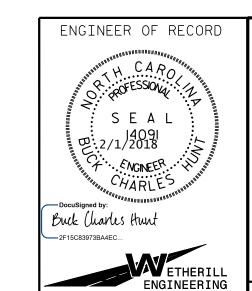
END BENT #1 SHOWN, END BENT #2 SIMILAR.



SKETCH SHOWING POINTS OF ATTACHMENT

* DENOTES GUARDRAIL ANCHOR ASSEMBLY

PROJECT NO. B-4775 MACON COUNTY STATION: 14+32.00 -L-



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STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD GUARDRAIL ANCHORAGE DETAILS

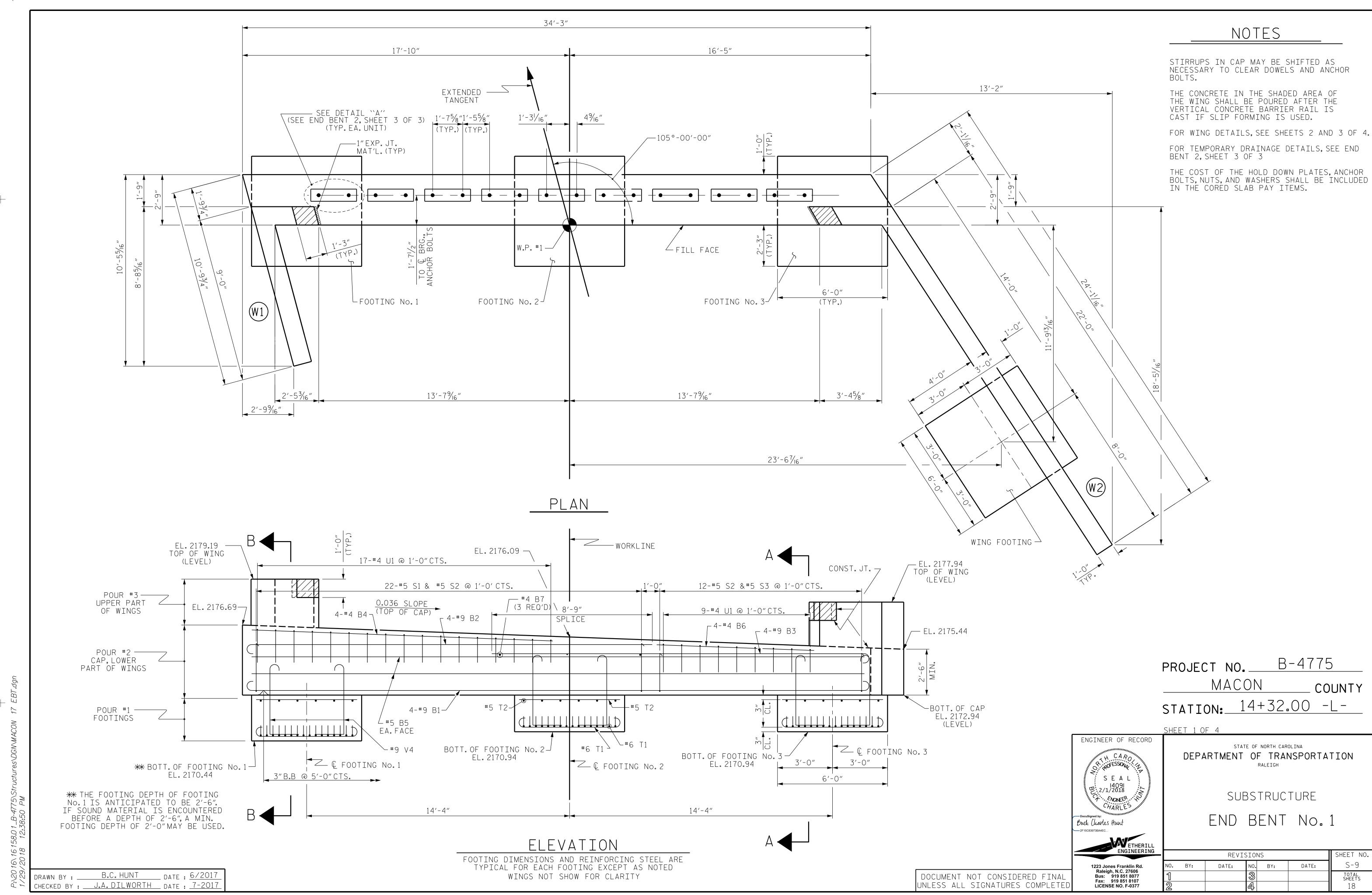
FOR VERTICAL CONCRETE BARRIER RAIL

REVISIONS SHEET NO. BY: DATE: NO. BY: TOTAL SHEETS

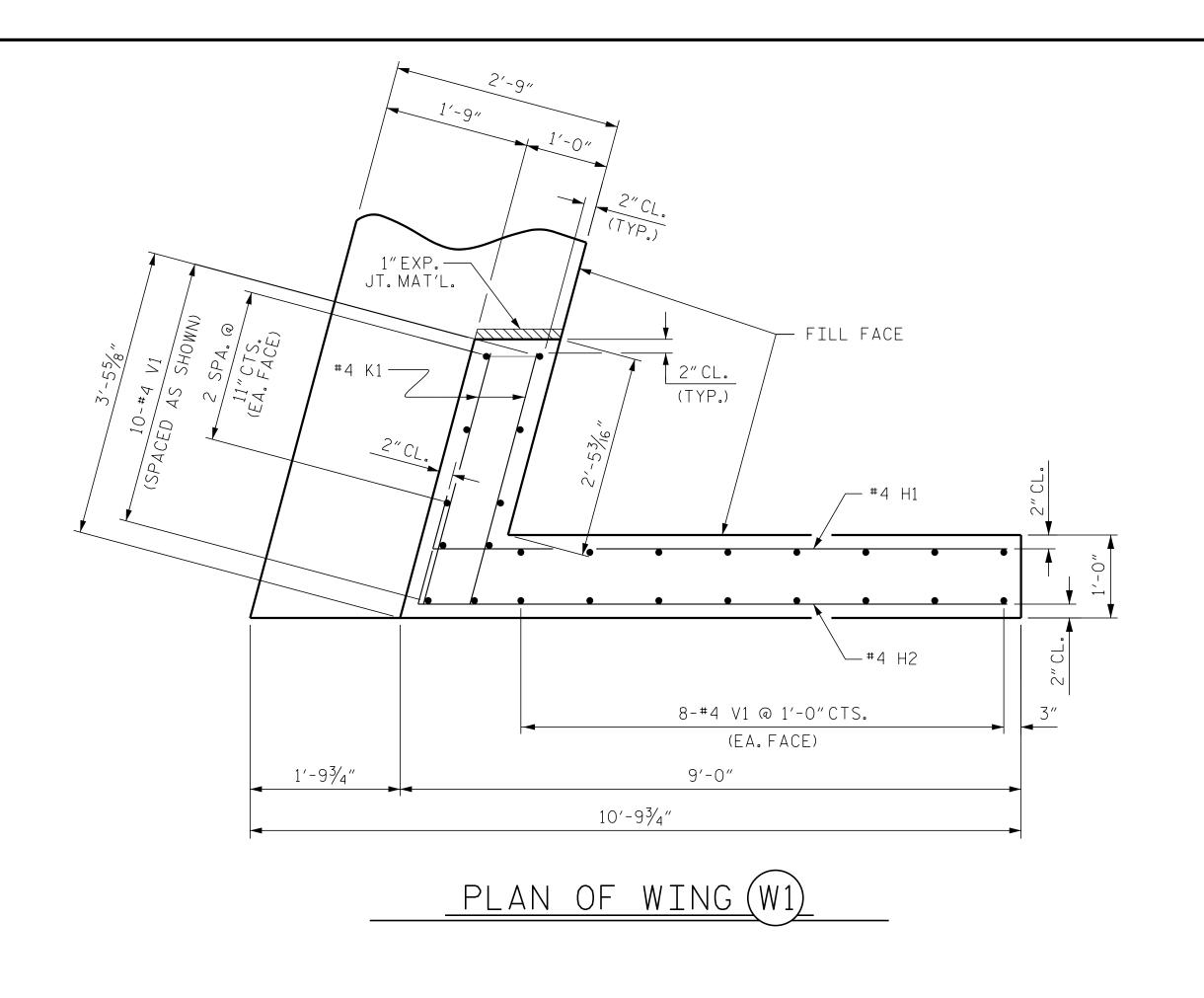
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETE

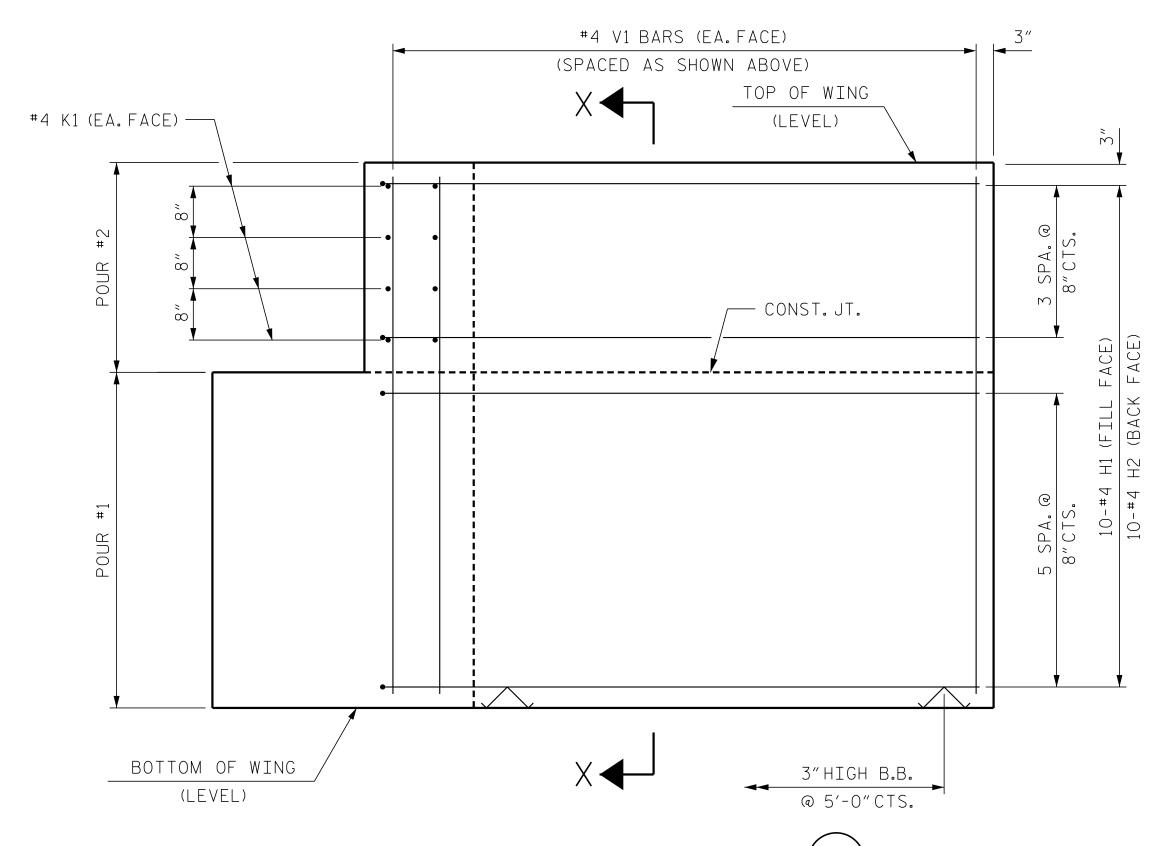
ASSEMBLED BY : J. PENDERGRAFT CHECKED BY: J. DILWORTH DATE: 1-17 DRAWN BY: MAA 5/10 MAA/GM CHECKED BY : GM 5/10

DATE: 1-17





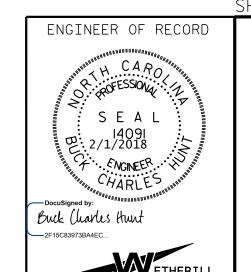




3 SPA. @ 8"CTS. FILL FACE 10-#4 H1 (EACH FACE) 10-#4 H2 (BACK FACE) CONST.JT. SECTION X-X

> PROJECT NO. B-4775 MACON COUNTY STATION: 14+32.00 -L-

SHEET 2 OF 4



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STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
RALEIGH

> SUBSTRUCTURE END BENT No. 1 WING DETAILS

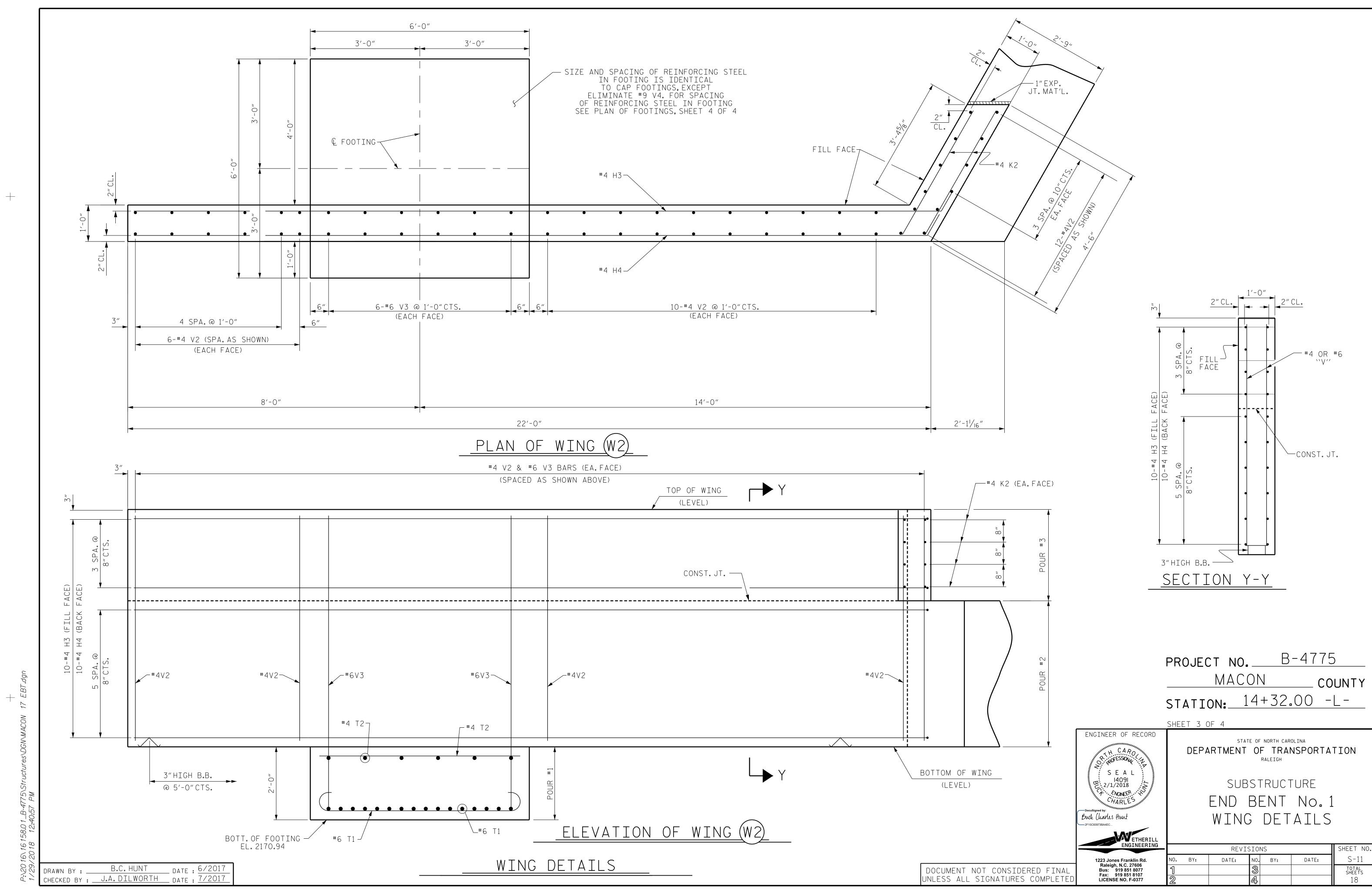
REVISIONS SHEET NO. DATE: NO. BY: DATE: BY: TOTAL SHEETS

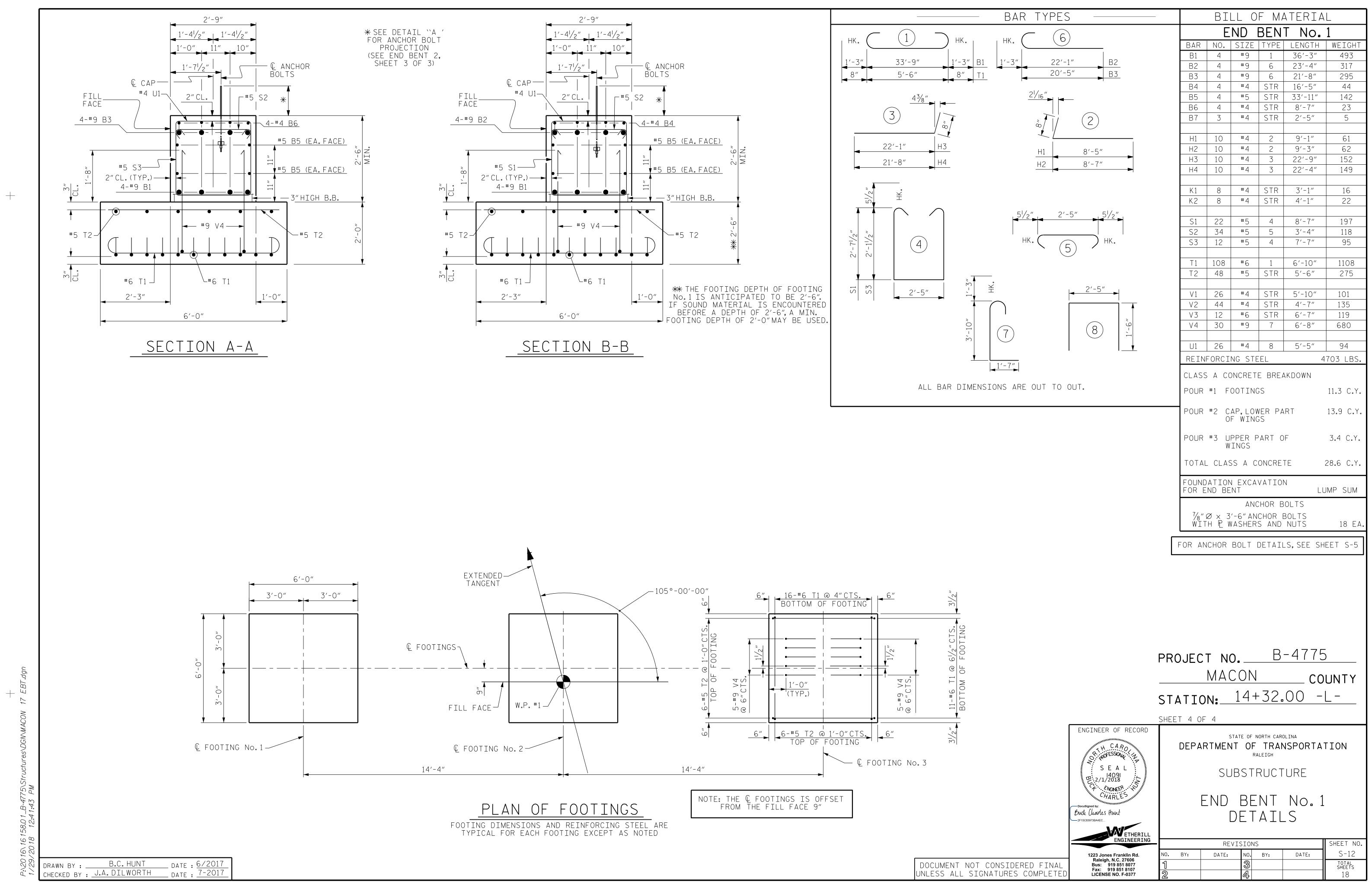
ELEVATION OF WING (W1)

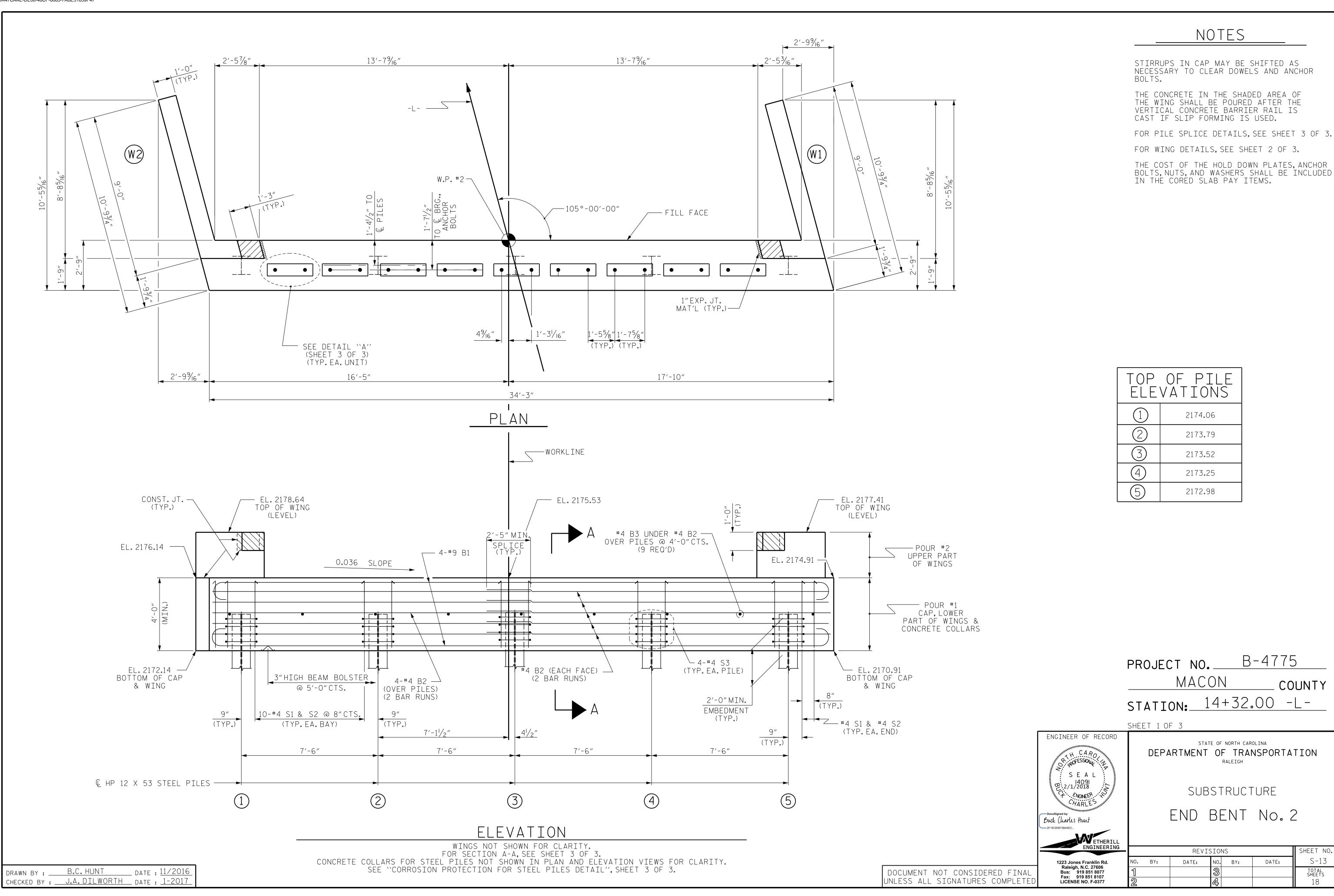
WING DETAILS

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

DRAWN BY: B.C. HUNT DATE: 11/2016
CHECKED BY: J.A. DILWORTH DATE: 7-2017





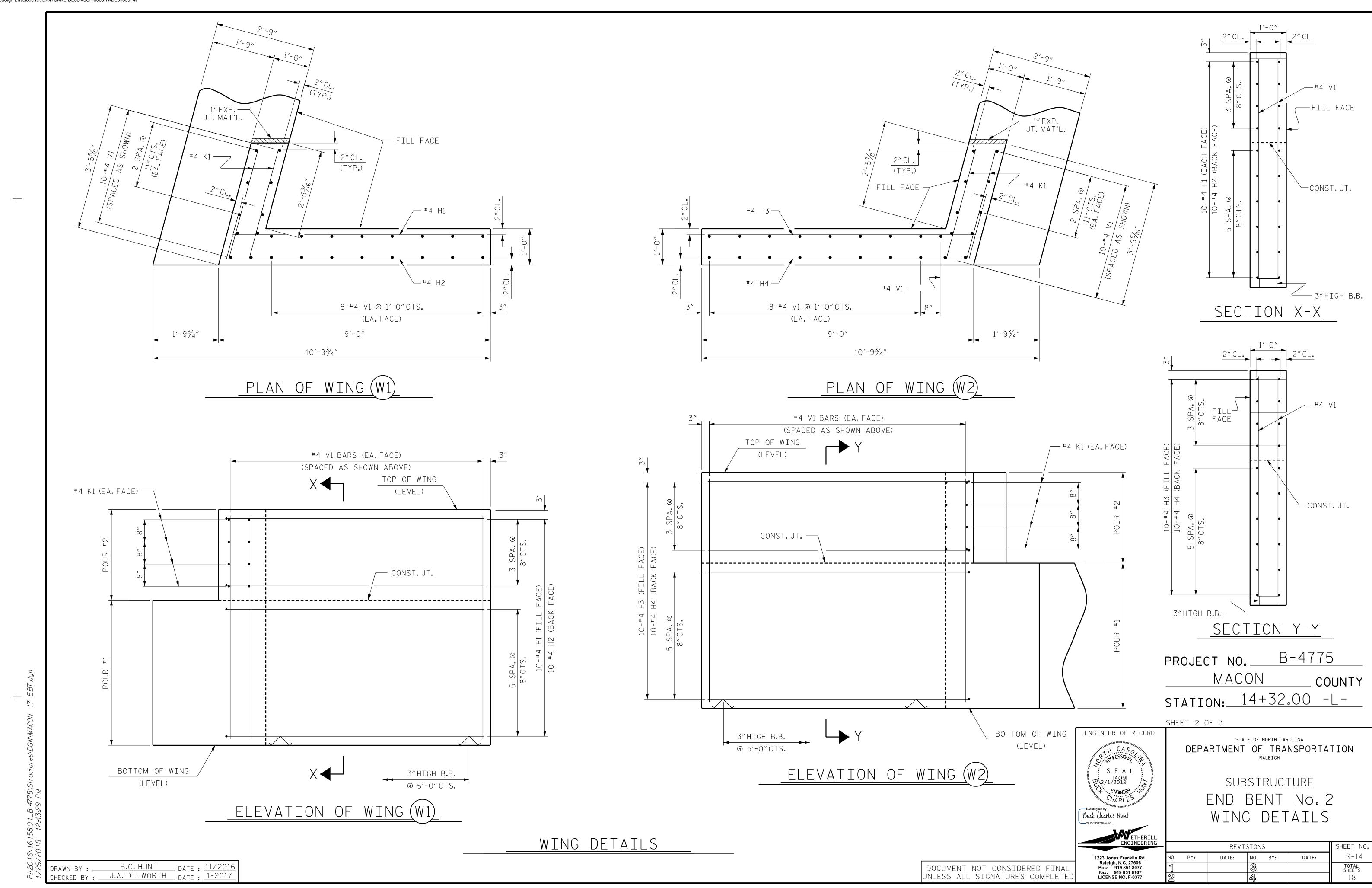


COUNTY

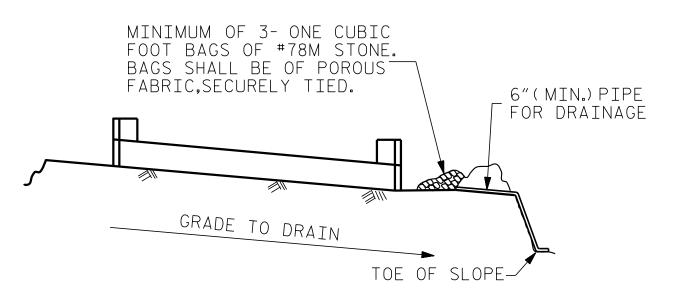
SHEET NO.

S-13

TOTAL SHEETS



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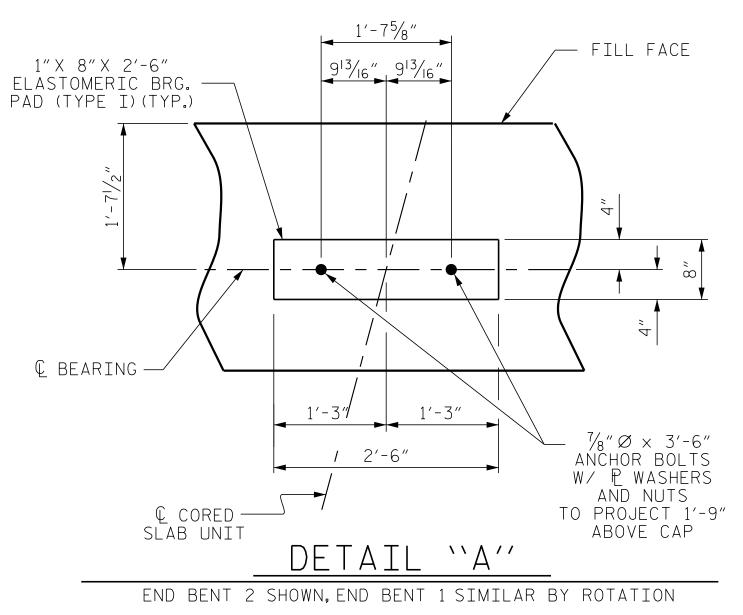


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT



/ BACK GOUGE DETAIL B ^PILE VERTICAL PILE HORIZONTAL OR VERTICAL √_T 0" T0 1/8" $0'' T0 \frac{1}{8}'$ DETAIL A DETAIL B POSITION OF PILE DURING WELDING.

PILE SPLICE DETAILS

33'-9" 28 B3 9 8'-5" 10 H2 8'-7" 10 H3 | 10 | 8'-10" H4 10 8'-8" K1 | 16 | 44 S<u>2</u> ı 44 S3 20 —1'-3'' LAP V1 | 53 2′-5″ (6)1'-8" Ø WINGS TOTAL CLASS A CONCRETE ALL BAR DIMENSIONS ARE OUT TO OUT.

BAR TYPES

END BENT No. 2 BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT #9 | 1 | 36'-3" #4 | STR | 18′-2″ 340 #4 | STR | 2'-5" 15 #4 9'-1" 61 #4 9′-3″ 62 #4 | 3 63 #4 9'-4" 62 33 #4 | STR | 3'-1" #4 10′-5″ 306 93 #4 3′-2″ #4 6′-6″ 87 #4 | STR | 6′-2″ 218 REINFORCING STEEL 2326 LBS CLASS A CONCRETE BREAKDOWN 17.2 C.Y. POUR #1 CAP, LOWER PART OF WINGS & COLLARS POUR #2 UPPER PART OF 2.1 C.Y.

BILL OF MATERIAL

HP 12 X 53 STEEL PILES NO: 5 LIN.FT.= 100 PILE DRIVING EQUIPMENT SET UP FOR HP 12 X 53 STEEL PILES 5 EA. 5 EA. STEEL PILE POINTS

19.3 C.Y.

3 EA. STEEL REDRIVES

ANCHOR BOLTS $\frac{7}{8}$ " \varnothing × 3'-6" ANCHOR BOLTS WITH P WASHERS AND NUTS

FOR ANCHOR BOLT DETAILS, SEE SHEET S-5

-BOTTOM OF CAP

SECTION A-A

(CONCRETE COLLAR NOT SHOWN FOR CLARITY. SEE "CORROSION PROTECTION FOR STEEL PILES DETAIL."

ROFESSIONA 1 SEAL 말: 1409l 2/1/2018 T. CACINEER. . V CHARLES Buck Charles Hunt WETHERILI

ENGINEER OF RECORD

HUNGER CARO

MACON COUNTY 14+32.00 -L-STATION:_

B-4775

SHEET 3 OF 3

PROJECT NO._

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SUBSTRUCTURE

END BENT No. 2 DETAILS

REVISIONS SHEET NO. S-15 NO. BY: DATE: BY: DATE: TOTAL SHEETS

CORROSION PROTECTION FOR STEEL PILES DETAIL

DOCUMENT NOT CONSIDERED FINAL

_ DATE : <u>11-2016</u> CHECKED BY : J.A. DILWORTH _ DATE : 1-2017

- FILL FACE

© PILES & — `CONCRETE COLLARS

2'-0" Ø CONCRETE COLLAR

(TYP.EACH PILE)

PLAN

CONCRETE-

COLLAR

© HP 12 X 53 STEEL PILE

ELEVATION

* SEE DETAIL ''A'' FOR ANCHOR BOLT PROJECTION

-4-#4 B2 @ 4" CTS.

OVER PILES

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1-#4 B2----EA.FACE #4 B3-#4 S1 ____

2-#9 B1 2"CL.(TYP.)—

FILL. FACE

4-#9 B1

2-#9 B1 © HP 12 X 53 — 3"HIGH B.B. STEEL PILE-

1'-4\\2" 1'-4\\2"

2'-9"

1'-0" 11" 10"

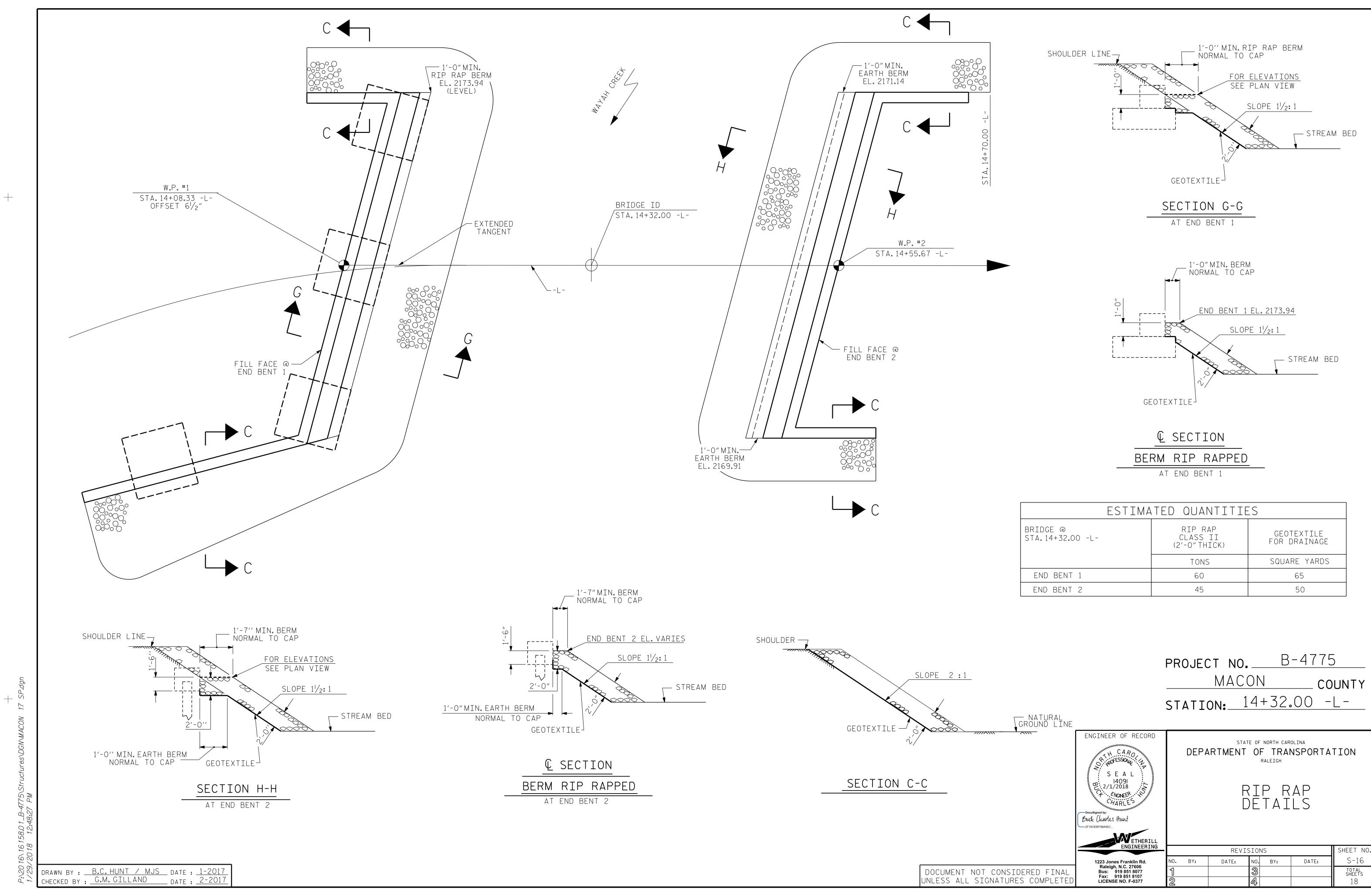
1'-71/2"

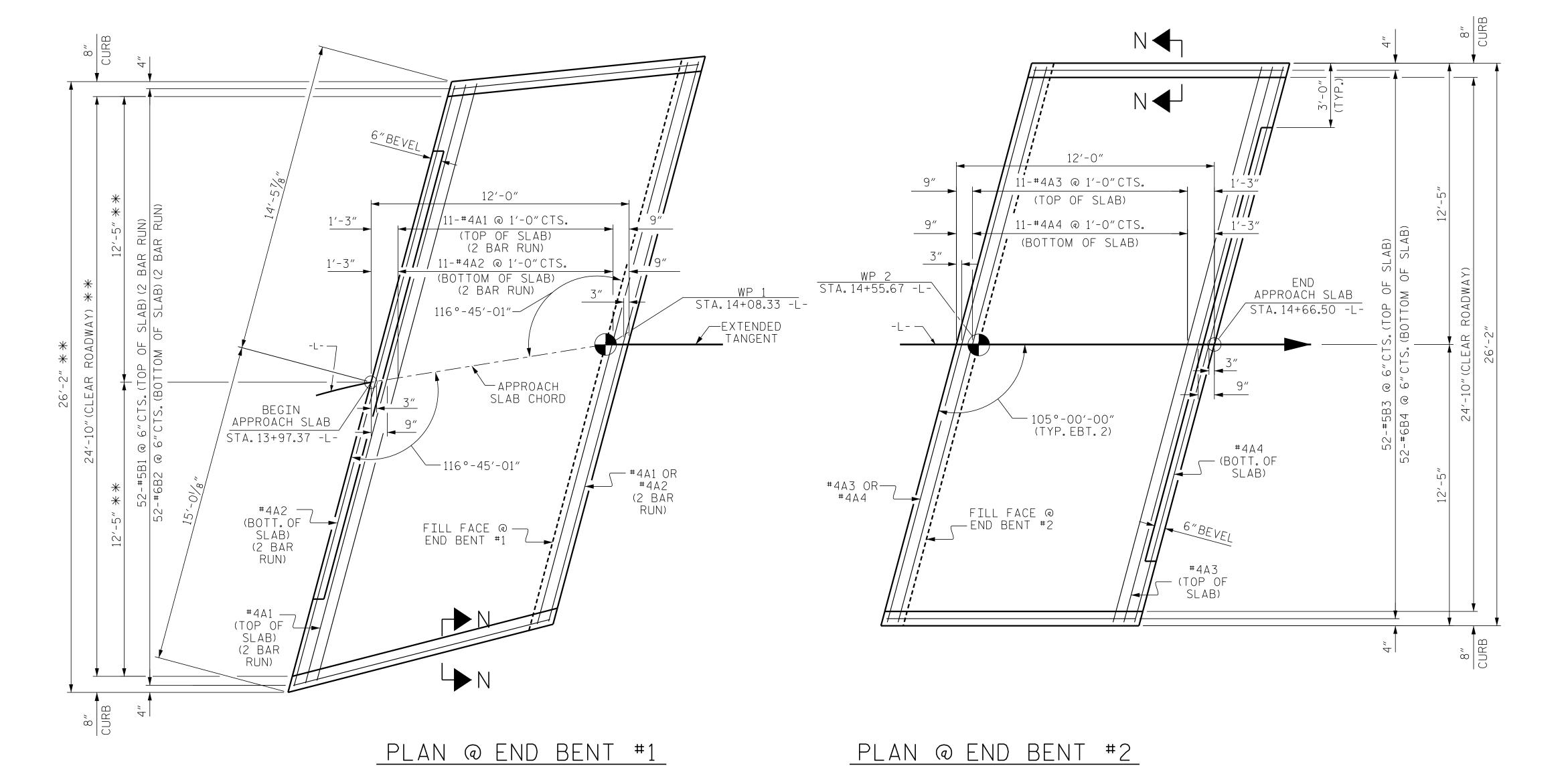
2″CL.

L ANCHOR

BOLTS

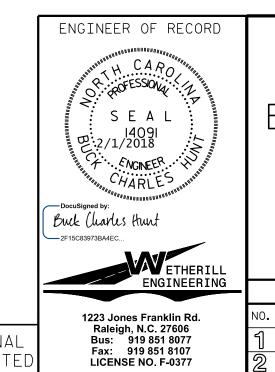
┌#4 S2 <u>*</u>|





** - RADIAL DIMENSION

PROJECT NO. B-4775 MACON COUNTY STATION: 14+32.00 -L-



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

BRIDGE APPROACH SLAB FOR PRESTRESSED CONCRETE CORED SLAB UNIT

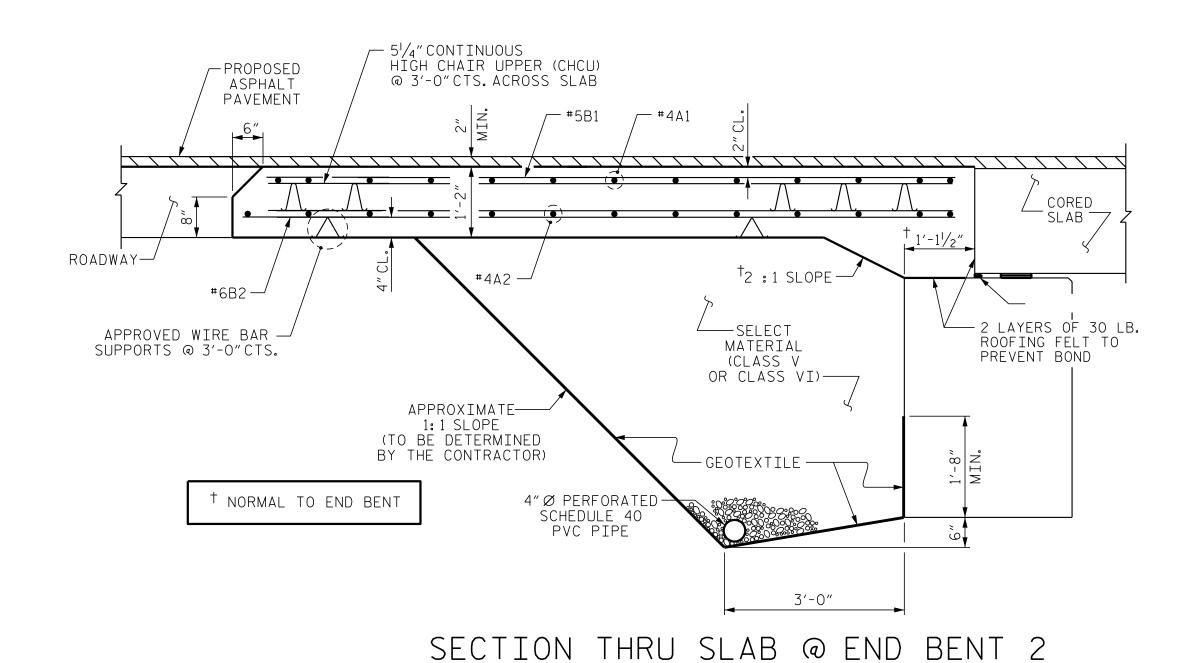
(SUB-REGIONAL TIER)

REVISIONS SHEET NO. S-17 NO. BY: DATE: BY: DATE: TOTAL SHEETS

DRAWN BY : <u>J. PENDERGRAFT</u> CHECKED BY : <u>B.C. HUNT</u> __ DATE : <u>1-17</u> __ DATE : <u>1-17</u>

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SECTION THRU SLAB @ END BENT 1



(TYPE II - MODIFIED APPROACH FILL)

NOTES

FOR BRIDGE APPROACH FILL INCLUDING GEOTEXTILE, 4" Ø DRAINAGE PIPE, AND SELECT MATERIAL BACKFILL, SEE ROADWAY PLANS.

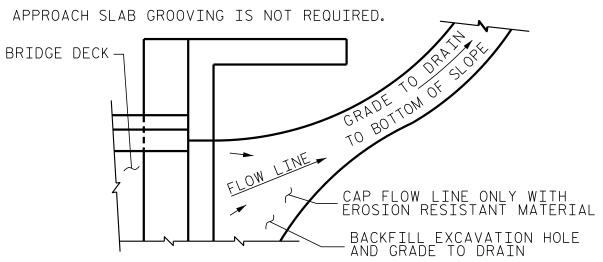
GEOTEXTILE SHALL BE TYPE 1 IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS SECTION 1056.

SELECT MATERIAL BACKFILL (CLASS V OR CLASS VI) SHALL BE IN

SELECT MATERIAL BACKFILL IS TO BE CONTINUOUS ALONG FILL FACE OF BACKWALL FROM OUTSIDE EDGE TO OUTSIDE EDGE OF APPROACH SLAB.

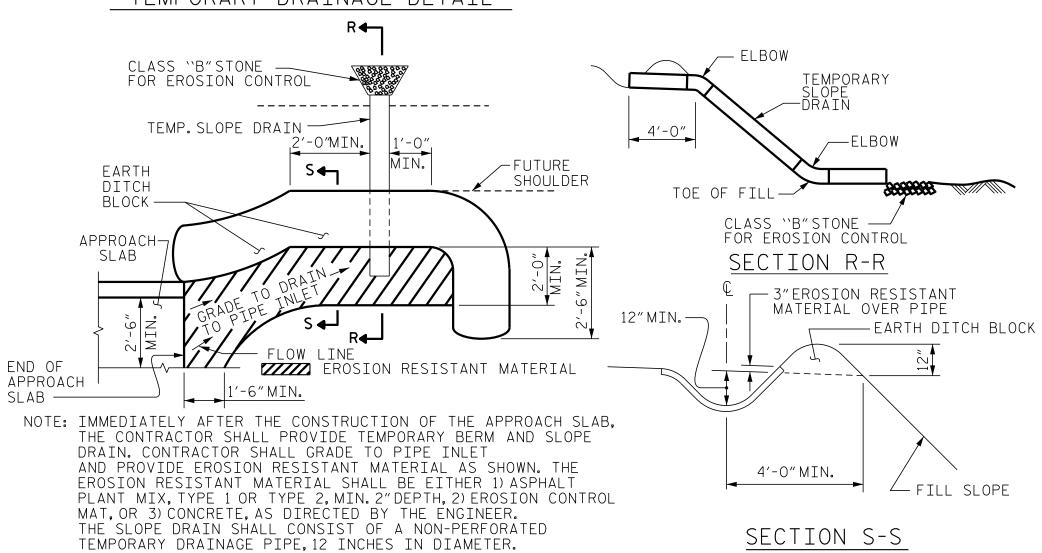
FOR THE 4" Ø DRAINAGE PIPE OUTLET(S), SEE ROADWAY STANDARD DRAWINGS. AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO

BE PAVED. SEE ROADWAY PLANS.



NOTE: IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

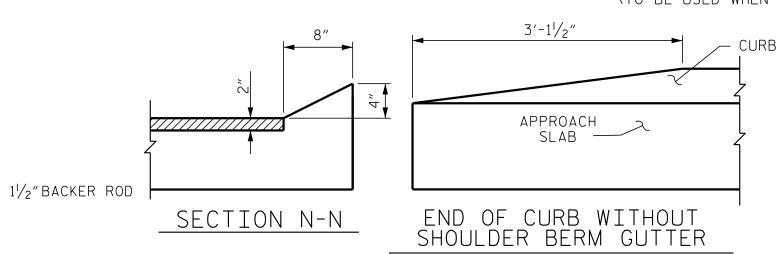
TEMPORARY DRAINAGE DETAIL



PLAN VIEW

TEMPORARY BERM AND SLOPE DRAIN DETAILS

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)



B-4775 PROJECT NO._ MACON COUNTY

BILL OF MATERIAL

APPROACH SLAB AT EB #1

BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT

APPROACH SLAB AT EB #2

BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT

268

1198

15.0

232

601

905

1137

LBS.

LBS.

C.Y.

LBS.

LBS.

C.Y.

* A1 | 26 | #4 | STR | 15'-7"

*B1 | 104 | #5 | STR | 8'-0"

REINFORCING STEEL

CLASS AA CONCRETE

REINFORCING STEEL

CLASS AA CONCRETE

REINFORCING STEEL

* EPOXY COATED

REINFORCING STEEL

* EPOXY COATED

B2 | 104| #6 | STR | 7'-8"

* A3 | 13 | #4 | STR | 26'-8" A4 | 13 | #4 | STR | 26'-8"

*B3 | 52 | #5 | STR | 11'-1"

B4 52 #6 STR 11'-7"

A2 | 26 | #4 | STR | 15'-5"

STATION: 14+32.00 -L-

SECTION S-S

CURB DETAILS

SPL:	ICE LE	NGTHS	
BAR SIZE	EPOXY COATED	UNCOATED	
#4	2'-0"	1'-9"	
#5	2'-6"	2'-2"	
#6	3′-10″	2'-7"	

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

BRIDGE APPROACH SLAB FOR PRESTRESSED CONCRETE CORED SLAB UNIT (SUB-REGIONAL TIER)

REVISION	1 S		SHEET NO.
DATE: NO.	BY:	DATE:	S-18
3			TOTAL SHEETS
4			18

DATE : <u>1-17</u> CHECKED BY : B.C. HUNT _ DATE : <u>1-17</u>

UNLESS ALL SIGNATURES COMPLETE

ACCORDANCE WITH STANDARD SPECIFICATIONS SECTION 1016.

DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL

DOCUMENT NOT CONSIDERED FINAL

H CAPO

ENGINEER OF RECORD

1223 Jones Franklin Rd. Raleigh, N.C. 27606 Bus: 919 851 8077 Fax: 919 851 8107

LICENSE NO. F-0377

BY:

STANDARD NOTES

DESIGN DATA:

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 11/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS.

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE $\frac{7}{8}$ " Ø SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - $\frac{7}{8}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF $\frac{7}{8}$ " Ø STUDS ALONG THE BEAM AS SHOWN FOR $\frac{3}{4}$ " Ø STUDS BASED ON THE RATIO OF 3 - $\frac{7}{8}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST \(\frac{1}{6}'' \) IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH